## Technical Memo

Date: Thursday, July 18, 2019<br>Project: US16 Corridor Study<br>To: Study Advisory Team<br>From: HDR<br>Subject: 2050 No-Build Conditions Traffic Operations

## Introduction

This memorandum presents the 2050 No-Build Conditions traffic operations analysis along the US16 corridor and other study area roadways for the US16 Corridor Study. The future-year NoBuild conditions scenario analyzes 2050 traffic forecasts with existing roadway conditions such as number of lanes, intersection traffic control, speed limits, etc. Traffic volumes represent traffic counts factored to a summer design season (June).

The purpose of this memorandum is to identify future traffic operational needs along the study corridor. Methodology used to develop traffic forecasts is presented in the Traffic Forecasts technical memorandum.

## Traffic Data

Traffic volumes presented in this technical memorandum reflect future-year traffic forecasts for a 2050 No-Build Conditions design season (June) as presented in the Traffic Forecasts technical memorandum. These forecasts are based on output from the Rapid City Area Metropolitan Planning Organization (RCAMPO) travel demand model and 2019 traffic counts. The following model versions were used to develop forecasts for this study:

- 2013 - base year
- 2040 - planning horizon

Intersection turning movements/percentages and heavy vehicle percentages (trucks, RVs, and lights pulling boats/cambers/trailers) were obtained from peak hour intersection turning movement counts obtained by the consultant team on Thursday, May 30, 2019. These counts were supplemented by SDDOT 24-hour roadway segment counts collected from Thursday, May 20, 2019, through the weekend. SDDOT segment counts were the basis for estimating free-flow speed. Peak hour factors (PHF) were adjusted to 0.90 within the urban area and 0.80 outside of the urban area (see Level of Service Goals section for discussion on urban and rural areas).

The intersection and roadway segment count locations are summarized in Table 1 and Table 2, respectively.

Table 1: Study Area Intersection Turning Movement Count Locations (Analysis Intersections)

| Ref \# | Street \#1 | Street \#2 |
| :---: | :---: | :---: |
| 1 | US16 | Cosmos Road |
| 2 | US16 | Beretta Road |
| 3 | US16 | Silver Mountain Road |
| 4 | US16 E | 16 E1 55.42 (Off-Ramp to Rockerville) |
| 5 | US16 W | Silver Mountain Road - Main Street - 16 WF 55.70 |
| 6 | US16 W | Pine Haven Drive / Private Road |
| 7 | US16 E | Rockerville Road / 16 EF 55.78 |
| 8 | US16 W | 16 W2 55.67 (On-Ramp from Rockerville) |
| 9 | US16 W | 16 W1 56.16 (Off-Ramp to Rockerville) |
| 10 | US16 E | Golden Hills Drive / 16 EF 55.93 |
| 11 | US16 W | Main Street / 16WF 55.70 |
| 12 | US16 E | 16 E2 56.09 (On-Ramp from Rockerville) |
| 13 | US16 | Strato Bowl Road |
| 14 | US16 | Strato Rim Drive |
| 15 | US16 | Busted Five Lane |
| 16 | US16 | Wilderness Canyon Road |
| 17 | US16 | Bear Country Exit |
| 18 | US16 | Bear Country Entrance |
| 19 | US16 | Croell Pit West/Main Entrance |
| 20 | US16 | Neck Yoke Road / Reptile Gardens South |
| 21 | US16 | Reptile Gardens Center |
| 22 | US16 | Reptile Gardens North |
| 23 | US16 | unknown road |
| 24 | US16 | Sammis Trail |
| 25 | US16 | Moon Meadows Drive |
| 26 | US16 | Addison Avenue |
| 27 | US16 | US16B / Catron Blvd |
| 28 | US16 | Tucker Street |
| 29 | US16 | Promise Road |
| 30 | US16 | Table Rock Road |
| 31 | US16 | Enchantment Road |
| 32 | US16 | Service Road / school entrances |
| 33 | US16 | Echo Ridge Drive |
| 34 | US16 | Fairmont Blvd / Cathedral Drive |
| 35 | US16B / Catron Blvd | Healing Way |
| 36 | Catron Boulevard | Les Hollers Way |
| 37 | US16B / Catron Blvd | Wellington Drive (west) |
| 38 | US16B / Catron Blvd | Wellington Drive (east) |
| 39 | Neck Yoke Road | Spring Creek Road / Rockerville Road |

See traffic volume figures for map.

Table 2: US16 Segment Count Locations

| Ref \# | MRM | US16 Segment Area Description |
| :---: | :---: | :---: |
| A | 55.00 | 2.5 miles east of US16/US16A <br> junction (Keystone Wye) |
| B | 63.00 | South of Moon Meadows Drive |
| C | 63.89 | Between Moon Meadows Drive and <br> US16B/Catron Blvd |
| D | 64.00 | Between Promise Road and <br> Tablerock Road |
| E | 64.90 | Between Tower Road and Echo <br> Ridge Drive |

See traffic volume figures for map.
Traffic data collected in eastbound and westbound directions at all locations.

## 2050 No-Build Conditions Traffic Forecasts

Traffic forecasts for 2045 were prepared using the most current version of the Rapid City Area MPO travel demand model (year 2040). Methodology used in the development of segment and intersection peak hour forecasts was consistent with NCHRP 765: Analytical Travel Forecasting Approaches for Project-Level Planning and Design.

Analysis traffic volumes for the 2050 No-Build Conditions are summarized in Figure 1. Traffic Forecasts technical memorandum presents more details regarding the methodology and process of developing future-year peak hour and daily traffic volumes.


| Legend |  |
| :---: | :---: |
| (0) Mileage Reference Marker (MRM) |  |
| Study Intersectio |  |
| 14,000 2050 Daily Traffic Volumes* |  |
| AM (PM) 2050 Peak Hour Traffic Volumes* |  |
| Notes: $\begin{aligned} & \text { Nolumes reffect June desion season }\end{aligned}$ |  |
| Stop-con | arem |



17. US16 \& Bear Country Exit

23. US16 \& Unknown Road

18. US16 \& Bear Country Ent.

19. US16 \& Croell Pit West Ent.

24. US16 \& Sammis Tr 39. Neck Yoke Rd \& Spring Creek Rd

20. US16 \& Neck Yoke Road

21. US16 \& Reptile Gardens Cent. 22. US16 \& Reptile Gardens (N)


AM - Early Morning Commute (Directional to Rapid City)
20. US16 \& Neck Yoke Road
24. US16 \& Sammis Tr
39. Neck Yoke Rd \& Spring Creek Rd


25. US16 \& Moon Meadows Rd

26. US16 \& Addison Avenue

32. US16 \& School Entrances

27. US16 \& US16B/Catron Blvd

33. US16 \& Echo Ridge Drive

28. US16 \& Tucker Street

34. US16 \& Cathedral Drive/ Fairmont Blvd

29. US16 \& Promise Road

30. US16 \& Table Rock Road


## Traffic Operations Analysis Methodology

Peak hour level of service (LOS) was calculated for study area intersections and roadway segments using Highway Capacity Software, Version 7 (HCS7) and methodology described in the $6^{\text {th }}$ Edition of the Highway Capacity Manual (HCM6).

## Intersection LOS

HCM6 analysis methodology measures intersection average control delay in terms of seconds of delay per vehicle (sec/veh) and applies a LOS value in accordance with thresholds presented in Table 3.

Table 3: Intersection Level of Service Thresholds

| LOS | Intersection Delay per Vehicle (sec/veh) |  |
| :---: | :---: | :---: |
|  | Signalized Intersections | Two-Way Stop-Control*, <br> All-Way Stop-Control, and <br> Roundabouts |
| A | $\leq 10$ | $\leq 10$ |
| B | $>10-20$ | $>10-15$ |
| C | $>20-35$ | $>15-25$ |
| D | $>35-55$ | $>25-35$ |
| E | $>55-80$ | $>35-50$ |
| F | Demand exceeds capacity; <br> $>80$ | Demand exceeds capacity; <br> $>50$ |

Source: Transportation Research Board, HCM6.

* Two-way stop-control LOS reflects worst-case stop-controlled approach.

Overall, or 'weighted', intersection delay was also calculated to present a second average delay measure at two-way stop-control intersections. This method accounts for the operational benefits afforded to the major, high volume through movements that are not stop or signalcontrolled at intersections. HCM6 reporting in HCS7 provides an average intersection delay value that reflects the weighted average delay of all vehicles entering the intersection. A LOS measure is applied to this overall intersection delay value using HCM6 All-Way Stop-Control LOS thresholds.

## Multilane Highway Segment LOS

HCM6 methodology was used to analyze multilane highway segments. HCM6 analysis methods measure lane density in terms of passenger cars per mile per lane ( $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ ) and applies a LOS value in accordance with thresholds presented in Table 4.

Table 4: Multilane Highway Segment Level of Service Thresholds

| LOS | Density (pc/mi/ln) |
| :---: | :---: |
| A | $\leq 11$ |
| B | $>11-18$ |
| C | $>18-26$ |
| D | $>26-35$ |
| E | $>35-45$ |
| F | Demand exceeds capacity OR <br> Density $>45$ |

## Merge and Diverge Segment LOS

For merge and diverge locations around the Rockerville area, HCM6 methodology for freeway merge and diverge segments was used to analyze similar locations around the Rockerville area. HCM analysis methods measure merge and diverge segment density in terms of passenger cars per mile per lane ( $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ ) and applies a LOS value in accordance with thresholds presented in Table 5.

Table 5: Freeway Merge and Diverge Segment Level of Service Thresholds

| LOS | Density (pc/mi/ln) |
| :---: | :---: |
| A | $\leq 10$ |
| B | $>10-20$ |
| C | $>20-28$ |
| D | $>28-35$ |
| E | $>35$ |
| F | Demand exceeds capacity |

## Level of Service Goals

The following minimum allowable LOS thresholds in Table 6 have been established for this study, applicable to the existing conditions.

Table 6: Minimum Allowable Level of Service by Facility and Area Type

| Facility Type | Minimum Allowable LOS |  | Notes |
| :---: | :---: | :---: | :---: |
|  | Rural Area | Urban Area |  |
| Signalized Intersections | LOS B | LOS C | Individual movements allowed to operate at |
| LOS D. |  |  |  |

Different LOS goals are identified for rural and urban areas. Currently, the urban-rural classification boundary is as follows:

- Urban - north of section line between Sammis Trail and Neck Yoke Road
- Rural - south of section line between Sammis Trail and Neck Yoke Road

Study LOS goals will be used to identify areas of operational needs along the corridor. Later in the study, these thresholds will be used to guide the development of potential improvements and subsequent evaluation of concepts.

## 2050 No-Build Conditions Traffic Operations Analysis

The 2050 No-Build Conditions traffic operations analysis reflects a scenario that analyzes forecasted 2050 volumes and existing roadway conditions, such as number of lanes, intersection traffic control, speed limits, etc. It is assumed that signal timings will be updated on a recurring basis in the future and thus this analysis reflects re-optimized timings.

Changes to the 2019 Existing Conditions roadway network typically center on developmentrelated modifications to accommodate this future development. Additional intersection legs to serve this development have been incorporated, per recommendations from traffic impact studies (TISs) to date, to help understand future-year needs from this development. The 2050 No-Build Conditions also assumes no redistribution or re-assignment of traffic due to congestion or existing traffic control, regardless of how unrealistic the resulting delay is at an intersection.

The other modification is at the intersection of US16B/Healing Way as both the existing stopcontrolled intersection and a future, planned signalized intersection is analyzed.

The 2050 No-Build Conditions intersection and highway segment operations are summarized in the following tables. HCS7 analysis reports are provided in the Appendix.

## Intersections

The intersections are grouped by urban and rural classifications, based on whether they are north or south of the section line between Sammis Trail and Neck Yoke Road. A graphical summary of intersection operations is presented in Figure 1.

## Urban Area Intersections

Table 7 through Table 10, provide operational results for the urban area TWSC and signalized intersections.

Because a traffic signal is currently planned for construction in 2019 at the US16B/Healing Way intersection, both the TWSC and signalized operations are shown in the tables.

Table 7: US16 Corridor Two-Way Stop-Control Intersection Traffic Operations - Urban Area

| US16 Corridor Intersection | Measure* | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Control Delay (sec/veh) | LOS | 95\% <br> Queue <br> (veh) | Control Delay (sec/veh) | LOS | 95\% <br> Queue <br> (veh) |
| Echo Ridge Drive | Overall | 0.9 | A | - | 0.7 | A | - |
|  | TWSC | 19.1 | C | 1.1 | 16.4 | C | 0.9 |
| Service Road / school entrances | Overall | 1.5 | A | - | 1.9 | A | - |
|  | TWSC | 49.7 | E | 6.0 | 44.3 | E | 1.8 |
| Enchantment Road | Overall | 11.0 | B | - | 98.0 | F | - |
|  | TWSC | 135.3 | F | 6.7 | 2716.4 | $F$ | 12.6 |
| Table Rock Road | Overall | 4.6 | A | - | 4.7 | A | - |
|  | TWSC | 93.6 | $F$ | 2.9 | 102.6 | $F$ | 2.8 |
| Promise Road | Overall | 33.4 | D | - | ~ | F | - |
|  | TWSC | 401.3 | F | 14.7 | $\sim$ | F | $\sim$ |
| Tucker Street | Overall | 0.6 | A | - | 0.9 | A | - |
|  | TWSC | 27.3 | D | 0.3 | 34.5 | D | 0.4 |
| Addison Avenue | Overall | 32.9 | D | - | ~ | F | - |
|  | TWSC | 960.6 | F | 10.1 | $\sim$ | F | $\sim$ |
| Moon Meadows Drive | Overall | 220.4 | F | - | $\sim$ | F | - |
|  | TWSC | 1664.1 | $F$ | 26.3 | $\sim$ | $F$ | ~ |
| Sammis Trail | Overall | 0.2 | A | - | 0.2 | A | - |
|  | TWSC | 18.4 | C | 0.2 | 27.4 | D | 0.4 |

[^0]Table 8: Other Study Area Two-Way Stop-Control Intersection Traffic Operations - Urban Area

| Intersection | Measure* $^{*}$ | AM <br> Control <br> Delay <br> (sec/veh) |  |  | LOS | 95\% <br> Queue <br> (veh) | Control <br> Delay <br> (sec/veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 95\% <br> Queue <br> (veh) |  |  |  |  |  |  |
| US16B/Catron Blvd <br> \& Healing Way <br> (signal planned 2019) | Overall | TWSC | 58.6 | F | - | $\mathbf{2 1 1 . 4}$ | F |
| US16B/Catron Blvd <br> \& Wellington Dr (W) | Overall | $\mathbf{0 . 4}$ | A | F | 3.6 | 3324.9 | $F$ |
|  | TWSC | 17.0 | $C$ | - | $\mathbf{0 . 2}$ | A | - |
| US16B/Catron Blvd <br> \& Wellington Dr (E) | Overall | 75.2 | F | - | $\sim$ | F | - |
|  | TWSC | 1443.9 | $F$ | 14.3 | $\sim$ | $F$ | $\sim$ |

* Measure notes: Overall intersection control delay represents the weighted average of each approach.

TWSC control delay represents the worst-cast stop-controlled approach delay and the associated $95^{\text {th }} \%$ queue.
~ Volume exceeds capacity on minor approaches and computation not defined.

Table 9: US16 Corridor Signalized Intersection Traffic Operations - Urban Area

| US16 Corridor <br> Intersection | Measure | AM |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LOS | Control <br> Delay <br> (sec/veh) | LOS | Notes |  |
| Fairmont Blvd / <br> Cathedral Drive | Signal | 24.3 | C | 34.7 | C | Measured queues exceed <br> available storage: SBL <br> (AM \& PM) and WBL (PM). |
| US16B / Catron Blvd | Signal | 75.4 | E | 136.6 | F | Multiple movements LOS <br> F in AM and PM peak <br> hours. Intersection <br> demand exceeds capacity. |

Table 10: Other Signalized Intersection Traffic Operations - Urban Area

| Intersection | Measure | AM |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Control <br> Delay <br> (sec/veh) | LOS | Control <br> Delay <br> (sec/veh) | LOS | Notes |  |
| Catron Blvd \& Les <br> Hollers Way | Signal | 79.6 | E | 55.0 | D | Measured queues exceed <br> available storage: NBL <br> (AM) and WBL (AM \& PM). <br> Multiple LOS E or $F$ <br> approaches. |
| US16B \& Healing Way <br> (signal planned 2019) | Signal | 23.9 | C | 33.9 | C |  |

[^1]There were five US16 corridor and US16B corridor TWSC intersections where the overall intersection LOS degraded to LOS D or worse:

- US16 \& Promise Road (AM and PM peak periods)
- US16 \& Addison Avenue (AM and PM peak periods)
- US16 \& Moon Meadows Drive (AM and PM peak periods)
- US16B/Catron Boulevard \& Healing Way (AM and PM peak periods)
- US16B/Catron Boulevard \& Wellington Drive (east) (AM and PM peak periods)

Each of these intersections are expected to see notable traffic demand from future development and have been studied extensively through recent traffic impact studies. Recommendations from those studies will be reviewed for consideration in the overall plan for the US16 corridor.

There are several other intersections throughout the study area that show LOS F on the stopcontrolled approach, but have an overall intersection LOS of A or B. These approaches demonstrate the increasing difficulty vehicles may have completing a turn onto or crossing US16 or US16B in the future. Collectively, the future-year demand at potentially signalized intersections is likely greater than what is shown in the 2050 No-Build Conditions forecasts. A signalized access point will attract traffic from adjacent stop-controlled intersections due to the difficulty in completing a turning or crossing movement.

At signalized intersections, the US16/US16B/Catron Boulevard intersection experiences LOS F in both the AM and PM peak periods as demand far exceeds capacity.

The Catron Boulevard and Les Hollers Way also shows degrading operations with LOS E and D in the AM and PM peak periods, respectively. Multiple approaches measure LOS E or F. Rightturn lanes would provide the greatest benefit at this intersection, particularly in the northbound direction, where a right-turn overlap could be used to improve capacity and lengthen green time for high-volume through movements. Traffic signal coordination of high volume through movements, in this case the westbound direction from US16/US16B/Catron Boulevard, would also benefit the intersection.

The US16 and Cathedral Drive/Fairmont Boulevard met LOS goals for this study, but queue spillback was noted on the dual southbound left-turn lanes and the westbound left-turn lane.

## Rural Area Intersections

The following tables, Table 11 through Table 13, provide operational results for the rural area TWSC intersections.

It was found that all rural area TWSC intersections meet study operational goals of weighted average intersection of LOS B or better. There are several locations, however, that exhibit worst-case stop-controlled approach LOS D, E or F, and are most prominent in the Neck Yoke Road area, Busted Five Lane area, and other common tourist-related exits.

Table 11: US16 Corridor Two-Way Stop-Control Intersection Traffic Operations - Rural Area (Neck Yoke Road Area to Strato Bowl Road)

| US16 Corridor Intersection | Measure* | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Control Delay (sec/veh) | LOS | 95\% Queue (veh) | Control Delay (sec/veh) | LOS | 95\% <br> Queue <br> (veh) |
| Unknown Road | Overall | 0.6 | A | - | 1.9 | A | - |
|  | TWSC | 42.5 | E | 0.4 | 78.0 | F | 1.3 |
| Reptile Gardens North | Overall | 3.1 | A | - | 64.0 | F | - |
|  | TWSC | 204.6 | F | 1.9 | 4539.5 | F | 6.3 |
| Reptile Gardens Center | Overall | 11.2 | B | - | 134.9 | F | - |
|  | TWSC | 577.9 | F | 4.0 | 4259.0 | F | 13.1 |
| Neck Yoke Road / Reptile Gardens S | Overall | 22.8 | C | - | 590.7 | F | - |
|  | TWSC | 525.4 | $F$ | 10.1 | 12975.8 | F | 23.1 |
| Croell Pit West / Main Entrance | Overall | 0.3 | A | - | 0.5 | A | - |
|  | TWSC | 39.3 | E | 0.3 | 63.4 | F | 0.6 |
| Bear Country Entrance | Overall | 0.3 | A | - | 0.3 | A | - |
|  | TWSC | 121.9 | $F$ | $\sim$ | 288.3 | $F$ | ~ |
| Bear Country Exit | Overall | 1.3 | A | - | 12.8 | B | - |
|  | TWSC | 40.6 | E | 2.0 | 368.1 | F | 8.4 |
| Wilderness Canyon Road | Overall | 2.5 | A | - | 1.0 | A | - |
|  | TWSC | 42.9 | E | 3.3 | 26.2 | D | 0.9 |
| Busted Five Lane | Overall | 0.8 | A | - | 3.6 | A | - |
|  | TWSC | 21.5 | C | 0.8 | 66.2 | $F$ | 4.1 |
| Strato Rim Drive | Overall | 0.6 | A | - | 1.4 | A | - |
|  | TWSC | 12.3 | B | 0.3 | 31.7 | D | 1.5 |
| Strato Bowl Road | Overall | 0.3 | A | - | 1.1 | A | - |
|  | TWSC | 20.7 | C | 0.2 | 25.3 | D | 0.9 |

[^2]Table 12: US16 Corridor Two-Way Stop-Control Intersection Traffic Operations - Rural Area (Rockerville Area to Keystone Wye)

| US16 Corridor Intersection | Measure* | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Control Delay (sec/veh) | LOS | 95\% Queue (veh) | Control Delay (sec/veh) | LOS | 95\% Queue (veh) |
|  <br> Golden Hills Drive | Overall | 0.6 | A | - | 0.6 | A | - |
|  | TWSC | 17.7 | C | 0.2 | 25.4 | D | 0.4 |
|  <br> Rockerville Road | Overall | 2.3 | A | - | 5.0 | A | - |
|  | TWSC | 20.0 | C | 0.8 | 48.1 | E | 3.4 |
| US16 W \& Pine Haven Drive | Overall | 0.8 | A | - | 0.8 | A | - |
|  | TWSC | 21.4 | C | 0.3 | 21.1 | C | 0.3 |
| US16 W \& Silver Mountain Road / Main Street | Overall | 0.7 | A | - | 0.7 | A | - |
|  | TWSC | 20.2 | C | 0.2 | 19.1 | C | 0.2 |
| Silver Mountain Road | Overall | 0.4 | A | - | 0.4 | A | - |
|  | TWSC | 25.2 | D | 0.4 | 30.7 | D | 0.5 |
| Beretta Road | Overall | 1.1 | A | - | 4.8 | A | - |
|  | TWSC | 57.8 | F | 0.8 | 212.0 | F | 3.6 |
| Cosmos Road | Overall | 1.8 | A | - | 2.4 | A | - |
|  | TWSC | 64.4 | F | 1.1 | 87.3 | F | 2.2 |

* Measure notes: Overall intersection control delay represents the weighted average of each approach.

TWSC control delay represents the worst-cast stop-controlled approach delay and the associated $95^{\text {th }} \%$ queue.
$\sim$ Volume exceeds capacity on minor approaches and computation not defined.

Table 13: Other Study Area Two-Way Stop-Control Intersection Traffic Operations - Rural Area

|  |  | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Intersection | Measure $^{\star}$ | $\begin{array}{c}\text { Control } \\ \text { Delay } \\ \text { (sec/veh) }\end{array}$ | LOS | $\begin{array}{c}\text { 95\% } \\ \text { Queue } \\ \text { (veh) }\end{array}$ | $\begin{array}{c}\text { Control } \\ \text { Delay } \\ \text { (sec/veh) }\end{array}$ | LOS | \(\left.\begin{array}{c}95\% <br>

Queue <br>
(veh)\end{array}\right]\)

[^3]
## Multilane Highway Segments

The US16 corridor was segmented in accordance with HCM6 methodology, with segment break points typically reflective of changes in grade. The respective segment was then analyzed based on 'level' or 'rolling' terrain or a specific grade for segments areas where long, steep grades were present (typically for segments longer than 0.25 miles and with grades of 3 percent or greater).

The US16 multilane highway segments are grouped by urban and rural classifications, based on whether they are north or south of the section line between Sammis Trail and Neck Yoke Road, in the following tables. A key that shows approximate limits of each segment is provided in the Appendix.

Overall, all US16 corridor multilane highway segments were measured to operate at LOS A or B with 2050 No-Build Conditions traffic volumes.

Table 14: US16 Corridor Multilane Highway Traffic Operations - Urban Area

| Seg. \# | Mainline | Approximate Limits |  | Approx. Length (miles) ${ }^{\star}$ | Analysis Grade (\%)** | AM LOS |  | PM LOS |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | From | To |  |  | EB | WB | EB | WB |
| 35 | US 16 | MRM 63.00 | Addison Ave | 0.8 | Rolling | B | B | B | B |
| 36 | US 16 | Addison Ave | US16B/ <br> Catron Blvd | 0.3 | 5.2 | B | A | B | B |
| 37 | US 16 | US16B/ Catron Blvd | MRM 66.00 | 1.5 | Rolling | B | A | B | B |
| $\begin{gathered} 38- \\ 39 \end{gathered}$ | US 16 | MRM 66.00 | Cathedral Dr/ Fairmont Blvd | 1.4 | 5.9 | B | B | B | C |

* Where multiple grades are shown, first length reflects eastbound direction and second length reflects westbound direction.
** Analysis grade reflects level, rolling or specific grade (segment upgrade typically shown for segments representing two directions of travel; grade may be shown as negative when US16 EB or WB analyzed as a downgrade when only one direction is analyzed due to alignment split), per HCM6 methodology.
Existing profile information obtained from SDDOT profile GIS layer (current spring 2019).
Note: limits and length are approximate, and thus may not align due to rounding and approximation of MRM locations.

Table 15: US16 Corridor Multilane Highway Traffic Operations - Rural Area (Keystone Wye to Busted Five Lane Area)

| Seg. \# | Mainline | Approximate Limits |  | Approx. Length (miles)* |  | Analysis Grade (\%)** | AM LOS |  | PM LOS |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | From | To |  |  | EB | WB | EB | WB |
| 1-2 | US 16 | Project Beginning MRM 50.75 | Cosmos Road | 0 |  |  | 4.5 | A | A | A | A |
| 3 | US 16 | Cosmos Road | MRM 52.00 | 0. |  | Rolling | A | B | B | A |
| 4 | US 16 | MRM 52.00 | MRM 52.50 | 0 |  | 4.0 | A | A | B | A |
| 5 | US 16 | MRM 52.50 | MRM 52.75 | 0. |  | 3.9 | A | A | A | A |
| 6 | US 16 | MRM 52.75 | MRM 53.00 | 0 |  | Level | A | A | A | A |
| 7 | US 16 | MRM 53.00 | Silver Mountain Rd | 0 |  | 5.0 | A | A | A | A |
| 8 | US 16 | Silver Mountain Rd | MRM 54.00 <br> (EB) / MRM <br> 53.75 (WB) | 0.7 | 0.6 | 3.1 | A | A | A | A |
| 9 | US 16 (EB) | MRM 54.00 | MRM 54.25 | 0.5 |  | Rolling | A | - | B | - |
| 10 | US 16 <br> (WB) | MRM 53.75 | MRM 54.00 | 0 |  | -3.5 | - | A | - | A |
| 11 | US 16 (EB) | MRM 54.00 | MRM 54.25 | 0. |  | 4.8 | - | A | - | A |
| 12 | US 16 | MRM 54.25 | MRM 54.50 | 0 |  | 6.0 | A | A | B | A |
| 13 | US 16 | MRM 54.50 | MRM 55.00 | 0 |  | 6.0 | A | A | A | A |
| 14 | US 16 (EB) | MRM 55.00 | MRM 55.25 | 0. |  | Level | A | - | A | - |
| 15 | US 16 <br> (WB) | MRM 55.00 | MRM 55.25 | 0. |  | 4.0 | - | A | - | A |
| 16 | US 16 | MRM 55.25 | MRM 55.75 (EB) / Silver Mtn Rd (WB) | 0.4 | 0.3 | 6.0 | A | A | A | A |
| 17 | US 16 | Silver Mountain Rd | Pine Haven Dr | 0 |  | 3.2 | - | A | - | A |
| 18 | US 16 | MRM 55.75 (EB)/ Pine Haven Rd (WB) | Golden Hills Dr (EB) / MRM 56.00 (WB) | 0.3 | 0.2 | Level | A | A | A | A |
| 19 | US 16 (EB) | Golden Hills Dr | MRM 56.50 | 0 |  | -5.7 | A | - | A | - |
| 20 | US 16 <br> (WB) | MRM 56.00 | Strato Bowl Rd | 0 |  | 3.0 | - | A | - | B |

[^4]Table 16: US16 Corridor Multilane Highway Traffic Operations - Rural Area (Busted Five Lane Area to Urban Area Boundary)

| Seg. \# | Mainline | Approximate Limits |  | Approx. Length (miles)* | Analysis Grade (\%)** | AM LOS |  | PM LOS |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | From | To |  |  | EB | WB | EB | WB |
| 21 | US 16 | MRM 56.50 (EB) / Strato Bowl Rd (WB) | MRM 57.00 | 0.50 .3 | Rolling | A | B | B | A |
| 22 | US 16 | MRM 57.00 | MRM 57.25 | 0.25 | 4.6 | A | A | B | B |
| 23 | US 16 | MRM 57.25 | MRM 57.50 | 0.25 | 6.0 | A | B | B | B |
| 24 | US 16 | MRM 57.50 | MRM 58.75 | 1.2 | Level | A | B | B | B |
| 25 | US 16 | MRM 58.75 | MRM 59.25 | 0.7 | 6.0 | A | B | B | B |
| 26 | US 16 | MRM 59.25 | Croell Pit West Entrance | 0.35 | Level | A | B | B | B |
| $\begin{gathered} 27 \\ 31 \end{gathered}$ | US 16 | Croell Pit West Entrance | MRM 61.50 | 1.8 | 6.0 | A | B | B | B |
| 32 | US 16 | MRM 61.50 | MRM 62.00 | 0.4 | Level | A | B | B | B |
| $\begin{gathered} 33 \\ 34 \end{gathered}$ | US 16 | MRM 62.00 | MRM 63.00 | 1.0 | 6.5 | A | B | B | B |

* Where multiple grades are shown, first length reflects eastbound direction and second length reflects westbound direction.
** Analysis grade reflects level, rolling or specific grade (segment upgrade typically shown for segments representing two directions of travel; grade may be shown as negative when US16 EB or WB analyzed as a downgrade when only one direction is analyzed due to alignment split), per HCM6 methodology.
Existing profile information obtained from SDDOT profile GIS layer (current spring 2019).
Note: limits and length are approximate, and thus may not align due to rounding and approximation of MRM locations.


## Merge and Diverge Segments

US16 merge and diverge locations around the Rockerville area were analyzed as freeway merge and diverge segments to give an indication to how these locations are operating in terms of vehicle density. Table 17 presents a summary of merge and diverge segment density. All locations were measured at LOS A or B.

Table 17: US16 Corridor Merge/Diverge Segment Traffic Operations - Rural Area (Rockerville Area)

|  |  | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Merge/ Diverge <br> Location | Segment <br> Type | LOS | Average <br> Density <br> (pc/mi/n) | Ramp <br> Influence <br> Area Density <br> (pc/mi/ln) | LOS | Average <br> Density <br> (pc/mi/ln) | Ramp <br> Influence <br> Area Density <br> (pc/mi/In) |
| US16 E \& 16 E1 <br> 55.42 <br> Off-ramp to Rockerville | Diverge | A | 7.9 | 7.1 | B | 11.2 | 10.4 |
| US16 W \& 16 W2 <br> 55.67 <br> On-ramp to Rockerville | Merge | B | 10.7 | 11.9 | B | 10.3 | 11.6 |
| US16 W \& WB <br> 55.70/Main Street <br> Off-ramp to Rockerville | Diverge | B | 10.9 | 10.9 | B | 10.7 | 10.7 |
| US16 E \& 16 E2 <br> 56.09 <br> On-ramp to Rockerville | Merge | A | 8.2 | 7.7 | B | 11.9 | 11.1 |
| US16 W \& 16 W1 <br> 56.15 <br> Off-ramp to Rockerville | Diverge | B | 10.9 | 10.9 | B | 10.9 | 10.9 |

## Conclusions

The 2050 No-Build Conditions analysis incorporates future development and build-out of the local network throughout the US16 corridor as identified in the 2050 Rapid City Area MPO travel demand model and TISs completed to date. The greatest impact of future development is around the US16/US16B/Catron Boulevard intersection where flat, developable land is available on all four intersection quadrants. Future build-out of the local network and new connections outside of the study area via Les Hollers Way (extended to Sheridan Lake Road) and Moon Meadows (possible extension to SD79) also contribute to increased demand along these routes.

The resulting 2050 No-Build Conditions traffic operations reflects the turning movement demand on various US16 and US16B intersections with the local roadway network and the need to accommodate these movements via safe and reliable access at some location along the corridor. TWSC intersections that resulted in an overall intersection LOS that does not meet study goals includes:

- Moon Meadows Drive
- Addison Avenue
- Promise Road
- Healing Way (traffic signal construction planned 2019)
- Wellington Drive (east)

Throughout the corridor, it is not unusual for TWSC intersections to operate at LOS F from the stop-controlled approach during peak hours. This typically does not necessitate a modification to traffic control or the intersection geometrics. However, LOS F from a TWSC does illustrate additional considerations that aid in the corridor study, such as:

- Provides an overall understanding of demand into/out of development or local network access locations.
- Helps guide an access management plan for the development of safe and operationally beneficial access locations.
- Provides an understanding of available gaps in traffic during a peak hour.
- Highlights the need and/or availability of alternate means of access to complete desired turning movements, such as the location of signalized intersections, local network connectivity to get to signalized intersections, and localized turning restrictions.

With regard to signalized intersections, the Catron Boulevard and Les Hollers Way intersection shows degrading operations with continued development and an extension of Les Hollers Way to Sheridan Lake Road. The inclusion of right-turn lanes, particularly for the northbound to eastbound right-turn movement, would help address capacity constraints at this intersection.

The combination of growing through traffic on both US16 and US16B continues to degrade operations at the US16/US16B/Catron Boulevard intersection. Demand in year 2050 is expected to exceed capacity with average vehicle delays around three minutes.



Notes:
Volumes reflect June design seaso
Stop-controlled intersections are two-way stop-control unless noted.


2050 NO-BUILD CONDITIONS TRAFFIC VOLUMES AND LEVEL OF SERVICE
17. US16 \& Bear Country Exit

23. US16 \& Unknown Road

18. US16 \& Bear Country Ent.

19. US16 \& Croell Pit West Ent.

24. US16 \& Sammis $\operatorname{Tr}$
39. Neck Yoke Rd \& Spring Creek Rd
20. US16 \& Neck Yoke Road

21. US16 \& Reptile Gardens Cent. 22. US16 \& Reptile Gardens (N)


AM - Early Morning Commute (Directional to Rapid City)
20. US16 \& Neck Yoke Road
24. US16 \& Sammis Tr
39. Neck Yoke Rd \& Spring Creek Rd


2050 NO-BUILD CONDITIONS TRAFFIC VOLUMES AND LEVEL OF SERVICE

25. US16 \& Moon Meadows Rd

26. US16 \& Addison Avenue

32. US16 \& School Entrances

27. US16 \& US16B/Catron Blvd

33. US16 \& Echo Ridge Drive

28. US16 \& Tucker Street

34. US16 \& Cathedral Drive/ Fairmont Blvd

29. US16 \& Promise Road


## Appendix

A. Multilane Highway Segmentation Key
B. HCS7 Reports

## Appendix A - Multilane Highway Segmentation Key

| $\begin{gathered} \text { Seg. } \\ \# \end{gathered}$ | Mainline | Approximate Limits |  | Approx. Length (miles) ${ }^{*}$ | Specific Grade (\%) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | From | To |  | EB | WB |
| 1 | US 16 | Project Beginning MRM 50.75 | MRM 51.00 | 0.3 | -4.1 | 4.1 |
| 2 | US 16 | MRM 51.00 | Cosmos Rd | 0.2 | -4.5 | 4.5 |
| 3 | US 16 | Cosmos Road | MRM 52.00 | 0.6 | Rolling | Rolling |
| 4 | US 16 | MRM 52.00 | MRM 52.50 | 0.5 | 4.0 | -4.0 |
| 5 | US 16 | MRM 52.50 | MRM 52.75 | 0.25 | -3.9 | 3.9 |
| 6 | US 16 | MRM 52.75 | MRM 53.00 | 0.2 | Level | Level |
| 7 | US 16 | MRM 53.00 | $\begin{gathered} \hline \text { Silver } \\ \text { Mountain Rd } \end{gathered}$ | 0.3 | -5.0 | 5.0 |
| 8 | US 16 | Silver <br> Mountain Rd | $\begin{aligned} & \text { MRM } 54.00 \\ & \text { (EB)/ MRM } \\ & 53.75 \text { (WB) } \end{aligned}$ | 0.70 .6 | -3.2 | 3.1 |
| 9 | US 16 (EB) | MRM 54.00 | MRM 54.25 | 0.54 | Rolling | - |
| 10 | $\begin{aligned} & \hline \text { US } 16 \\ & \text { (WB) } \end{aligned}$ | MRM 53.75 | MRM 54.25 | 0.4 | - | -3.5 |
| 11 | US 16 (EB) | MRM 54.00 | MRM 54.25 | 0.25 | - | 4.8 |
| 12 | US 16 | MRM 54.25 | MRM 54.50 | 0.3 | 6 | -4.5 |


| $\begin{gathered} \text { Seg. } \\ \# \end{gathered}$ | Mainline | Approximate Limits |  | Approx. <br> Length <br> (miles) ${ }^{*}$ | Specific Grade (\%) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | From | To |  | EB | WB |
| 13 | US 16 | MRM 54.50 | MRM 55.00 | 0.3 | -7 | 6 |
| 14 | US 16 (EB) | MRM 55.00 | MRM 5.25 | 0.25 | Level | - |
| 15 | $\begin{aligned} & \text { US } 16 \\ & \text { (WB) } \end{aligned}$ | MRM 55.00 | MRM 55.25 | 0.25 | - | 3.8 |
| 16 | US 16 | MRM 55.25 | MRM 55.75 (EB) / Silver Mtn Rd (WB) | 0.4 0.3 | -6 | 6 |
| 17 | $\begin{aligned} & \text { US } 16 \\ & \text { (WB) } \end{aligned}$ | Silver Mountain Rd | Pine Haven Dr | 0.2 | - | 3.2 |
| 18 | US 16 | MRM 55.75 (EB)/ Pine Haven Rd (WB) | Golden Hills Dr (EB) / MRM 56.00 (WB) | 0.30 .2 | Level | Level |
| 19 | US 16 (EB) | Golden Hills Dr | MRM 56.50 | 0.3 | -5.7 | - |
| 20 | $\begin{aligned} & \hline \text { US } 16 \\ & \text { (WB) } \end{aligned}$ | MRM 56.00 | $\begin{aligned} & \hline \text { Strato Bowl } \\ & \text { Rd } \end{aligned}$ | 0.5 | - | 3.0 |


| $\begin{gathered} \text { Seg. } \\ \# \end{gathered}$ | Mainline | Approximate Limits |  | Approx. <br> Length <br> (miles) ${ }^{\text {² }}$ | Specific Grade (\%) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | From | To |  | EB | WB |
| 21 | US 16 | $\begin{aligned} & \text { MRM } 56.50 \\ & \text { (EB) / Strato } \end{aligned}$ Bowl Rd (WB) | MRM 57.00 | 0.5 0.3 | Rolling | Rolling |
| 22 | US 16 | MRM 57.00 | MRM 57.25 | 0.25 | 4.6 | -4.6 |
| 23 | US 16 | MRM 57.25 | MRM 57.50 | 0.25 | -6 | 6 |
| 24 | US 16 | MRM 57.50 | MRM 58.75 | 1.2 | Level | Level |
| 25 | US 16 | MRM 58.75 | MRM 59.25 | 0.7 | -6 | 6 |

per HCMS methodology.
Existing profili information obtoined from sooot profile Gis layer (current spring 2019)

## Legend

Segment Boundary
35 Segment Number

US Highway 16 comaonstuor


| $\begin{gathered} \text { Seg. } \\ \# \end{gathered}$ | Mainline | Approximate Limits |  | Approx. Length (miles) ${ }^{*}$ | Analysis Grade (\%) | Specific Grade (\%) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | From | To |  |  | EB | WB |
| 26 | US 16 | MRM 59.25 | Croell Pit West Entrance | 0.35 | Level | Level | Level |
| 27 | US 16 | Croell Pit West Entrance | MRM 60.25 | 0.5 | 6.0 | -6 | 6 |
| 28 | US 16 | MRM 60.25 | MRM 60.50 | 0.2 | 4.7 | -4.7 | 4.7 |
| 29 | US 16 | MRM 60.50 | MRM 61.00 | 0.7 | 6.0 | -6 | 6 |
| 30 | US 16 | MRM 61.00 | MRM 61.25 | 0.2 | 5.0 | -5 | 5 |
| 31 | US 16 | MRM 61.25 | MRM 61.50 | 0.2 | 5.8 | -5.8 | 5.8 |
| 32 | US 16 | MRM 61.50 | MRM 62.00 | 0.4 | Level | Level | Level |
| 33 | US 16 | MRM 62.00 | MRM 62.25 | 0.4 | 6.5 | 6.5 | -6.5 |
| 34 | US 16 | MRM 62.25 | MRM 63.00 | 0.6 | 4.3 | 4.3 | -4.3 |


| Seg. | Mainline | Approximate Limits |  | Approx. Length (miles) ${ }^{*}$ | Specific Grade (\%) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | From | To |  | EB | WB |
| 35 | US 16 | MRM 63.00 | Addison Ave | 0.8 | Rolling | Rolling |
| 36 | US 16 | Addison Ave | $\begin{gathered} \text { US16B/ } \\ \text { Catron Blvd } \end{gathered}$ | 0.3 | -5.2 | 5.2 |
| 37 | US 16 | US16B/ <br> Catron Blvd | MRM 66.00 | 1.5 | Rolling | Rolling |
| 38 | US 16 | MRM 66.00 | Echo Ridge Dr | 0.8 | -5.9 | 5.9 |
| 39 | US 16 | Echo Ridge Dr | Cathedral Dr/ Fairmont Blvd | 0.6 | -5.3 | 5.3 |

## Legend

** Anolysis grade reflects level, rolling or specific grode (segment upgrade typically shown for segments representing two directions of trovel), per HCM6 methoodology.
Existing profili information obtoined from SODOT profile GIS laver (current spring 2019)
Note: :Iimits ond length ore opproximate, and thus may not align due to rounding and approximation of MEM locations.

## Appendix B - HCS7 Reports


[^0]:    * Measure notes: Overall intersection control delay represents the weighted average of each approach.

    TWSC control delay represents the worst-cast stop-controlled approach delay and the associated $95^{\text {th }} \%$ queue.
    ~ Volume exceeds capacity on minor approaches and computation not defined.

[^1]:    US16B and Healing Way signal planned for 2019 installation.

[^2]:    * Measure notes: Overall intersection control delay represents the weighted average of each approach.

    TWSC control delay represents the worst-cast stop-controlled approach delay and the associated $95^{\text {th }} \%$ queue.
    ~ Volume exceeds capacity on minor approaches and computation not defined.

[^3]:    * Measure notes: Overall intersection control delay represents the weighted average of each approach.

    TWSC control delay represents the worst-cast stop-controlled approach delay and the associated 95 ${ }^{\text {th }} \%$ queue.
    $\sim$ Volume exceeds capacity on minor approaches and computation not defined.

[^4]:    * Where multiple grades are shown, first length reflects eastbound direction and second length reflects westbound direction.
    ${ }^{* *}$ Analysis grade reflects level, rolling or specific grade (segment upgrade typically shown for segments representing two directions of travel; grade may be shown as negative when US16 EB or WB analyzed as a downgrade when only one direction is analyzed due to alignment split), per HCM6 methodology.
    Existing profile information obtained from SDDOT profile GIS layer (current spring 2019).
    Note: limits and length are approximate, and thus may not align due to rounding and approximation of MRM locations.

